

GENERAL NOTES

A. GENERAL INFORMATION

1.

READ STRUCTURAL DOCUMENTS IN CONJUNCTION WITH CONTRACT DOCUMENTS, WHICH INCLUDE, BUT ARE NOT LIMITED TO, ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DOCUMENTS.
2.

CONTRACTOR TO BE RESPONSIBLE FOR CHECKING SITE CONDITIONS AGAINST DOCUMENTS, BEFORE PROCEEDING WITH THE WORK, AND REPORT DISCREPANCIES TO THE CONSULTANT.
3.

CONTRACTOR TO PROVIDE LABOUR, MATERIALS, AND EQUIPMENT TO COMPLETE ALL STRUCTURAL WORK INDICATED.
4.

CARRY OUT CONSTRUCTION OPERATIONS, INCLUDING THE INSTALLATION OF TEMPORARY GUYING AND SHORING REQUIRED, ENSURING THAT THE EXISTING STRUCTURE OR MEMBERS ALREADY ERECTED ARE NOT LOADED IN EXCESS OF THEIR SAFE LOAD CARRYING CAPACITY.
5.

STRUCTURAL DOCUMENTS DO NOT NECESSARILY SHOW ALL OPENINGS AND SLAB VARIATIONS REQUIRED. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR THE EXACT LOCATION, NUMBER, AND SIZE OF OPENINGS, TRENCHES, PITS, SUMPS, SLEEVES, AND DEPRESSIONS. PROVIDE STRUCTURAL FRAMING AT THESE LOCATIONS IN ACCORDANCE WITH THE APPLICABLE TYPICAL DETAIL.

B. REFERENCE STANDARDS/CODES AND ACTS

1.

CONFORM WITH THE 2012 BUILDING CODE (ONTARIO REGULATION 350/12), AND ANY APPLICABLE ACTS OF ANY AUTHORITY HAVING JURISDICTION, AND THE FOLLOWING:

1.1.

CAN/CSA A23.1 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION.

1.2.

CAN/CSA A23.2 METHODS OF TEST FOR CONCRETE.

1.3.

CAN/CSA A23.3 DESIGN OF CONCRETE STRUCTURES.

1.7.

RSIC REINFORCING STEEL INSTITUTE OF CANADA (RSIC), MANUAL OF STANDARD PRACTICE.

1.8.

O86 ENGINEERING DESIGN IN WOOD (LIMIT STATES DESIGN).

1.9.

CSA G30.18 CARBON STEEL BARS FOR CONCRETE REINFORCING.
2.

ALL STANDARDS AND PUBLICATIONS REFERENCED BY THE STANDARDS NOTED ABOVE ARE TO APPLY.
3.

WHERE THERE ARE DIFFERENCES BETWEEN THE DOCUMENTS AND THE STANDARDS, CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN.

D. MATERIALS

1.

PROVIDE ONLY NEW STRUCTURAL MATERIALS IN ACCORDANCE WITH THE REFERENCE STANDARDS AND THE FOLLOWING, UNLESS OTHERWISE NOTED.

1.1.

CONCRETE:

1.1.1.

EXPOSED TO WEATHER: F'c = 35 MPa AT 28 DAYS, SLUMP 80mm (3"), EXPOSURE CLASS C-1, W/C RATIO 0.40, AIR CONTENT 5%-8%, AND CONCRETE TO HAVE A MINIMUM CEMENTING MATERIAL CONTENT OF 320 kg/m³.

1.2.

REINFORCING STEEL: CONFORM TO CSA G30 SERIES, GRADE 400.

1.3.

STRUCTURAL BOLTS, NUTS AND WASHERS: CONFORM TO ASTM A325M.

1.4.

SAWN LUMBER: SPRUCE – PINE – FIR (S-P-F), NO. 1/2 GRADE OR BETTER UNLESS NOTED ON DRAWINGS. CONFORM TO CSA-0141. ALL EXTERIOR LUMBER TO BE PRESSURE TREATED.

E. EXECUTION

1.

FOUNDATIONS

1.1.

FOUND ALL FOOTINGS ON SOIL CAPABLE OF SUSTAINING A MINIMUM ULTIMATE LIMIT STATES/ SERVICE LIMIT STATES BEARING STRESS (ULS/SLS) OF 75 kPa / 50 kPa.

1.2.

FOUND ALL FOOTINGS WHICH WILL BE EXPOSED TO FROST ACTION IN THE COMPLETED BUILDING A MINIMUM OF 1200 mm (4'-0") BELOW FINISHED GRADE.

1.3.

DO NOT EXCEED A RISE OF 7 IN A RUN OF 10 IN THE LINE OF SLOPE BETWEEN ADJACENT FOOTING EXCAVATIONS OR ALONG STEPPED FOOTINGS. FOR STEPPED FOOTINGS, USE STEPS NOT EXCEEDING 600 mm (2'-0") IN HEIGHT AND 1200 mm (4'-0") (MIN.) IN LENGTH.

1.4.

SOIL BEARING CAPACITY SPECIFIED MUST BE VERIFIED BY THE SOIL ENGINEER PRIOR TO THE PLACING OF FOOTINGS AND ANY NON-CONFORMANCE WITH THE SPECIFIED MINIMUM CAPACITIES MUST BE IMMEDIATELY REPORTED TO THE STRUCTURAL ENGINEERS.
2.

CONCRETE

2.1.

CONSTRUCTION JOINTS FOR WALLS, SLABS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION. GENERALLY JOINTS IN SLABS SHALL BE AT RIGHT ANGLES TO THE SPANS, AT MID-SPAN IF POSSIBLE AND BE CLEAR OF SUPPORTS AND POINT LOADS.

2.2.

WHEN ATMOSPHERIC TEMPERATURE IS AT OR BELOW 5°C OR WHEN THERE IS A POSSIBILITY OF IT FALLING TO THAT LIMIT, PLACE CONCRETE IN ACCORDANCE WITH THE REQUIREMENTS OF CAN/CSA A23.1 "COLD WEATHER CONCRETING" AND ACI 306 "RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING". WHEN ATMOSPHERIC TEMPERATURE IS AT OR ABOVE 27°C PLACE CONCRETE IN ACCORDANCE WITH CAN/CSA A23.1 "HOT WEATHER CONCRETING" AND ACI 305 "RECOMMENDED PRACTICES FOR HOT WEATHER CONCRETING".

5.

TIMBER FRAMING

5.1.

ALL FRAMING, BRIDGING, NAILING, PROTECTION, HARDWARE AND OTHER FRAMING DETAILS ARE TO BE IN ACCORDANCE WITH PART 9 OF THE ONTARIO BUILDING CODE, LATEST EDITION.

5.2.

UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS, THE CONTRACTOR SHALL PROVIDE STANDARD SIMPSON STRONG TIE HARDWARE OR APPROVED EQUIVALENT FOR ALL JOIST HANGERS, BEAM HANGERS, BEAM SEATS, POST ANCHORS, ETC.

5.3.

MEMBERS SHALL BE ALIGNED LEVEL AND PLUMB, WITHIN A TOLERANCE OF 1 IN 500.

5.4.

MAKE ADEQUATE PROVISIONS FOR ERECTION STRESSES AND FOR SUFFICIENT TEMPORARY BRACING TO KEEP THE STRUCTURAL FRAME PLUMB AND IN TRUE ALIGNMENT UNTIL THE COMPLETION OF THE ENTIRE FRAMING INCLUDING INSTALLATION OF THE FLOOR AND WALL SHEATHING.

5.5.

FRAME AROUND ALL OPENINGS WITH DOUBLE HEADERS AND TRIMMERS NAILED TOGETHER WITH TWO ROWS OF 89 MM (3-1/2 ") SPIRAL NAILS AT 200 MM C/C (8" C/C) STAGGERED UNLESS NOTED OTHERWISE. DO NOT SPLICE MEMBERS BETWEEN SUPPORTS.

5.6.

ALL BUILT UP BEAMS TO BE NAILED TOGETHER WITH TWO ROWS OF 89 MM (3-1/2 ") SPIRAL NAILS AT 200 MM (8") C/C, STAGER ROWS TOP AND BOTTOM. DO NOT SPLICE MEMBERS BETWEEN SUPPORTS.

5.7.

PROVIDE SOLID BLOCKING BETWEEN JOISTS OVER SUPPORT AT ALL CANTILEVERED CONDITIONS.

5.8.

5.16 PROVIDE 38x38 (2"x2") DIAGONAL CROSS BRIDGING AT MAXIMUM 2,100 MM (6'-11") C/C UNLESS NOTED OTHERWISE, FOR ALL SAWN JOIST LOCATIONS.

5.17.

PROVIDE MINIMUM BEARING OF 50 MM (2") FOR ALL JOISTS.

5.18.

PROVIDE MINIMUM BEARING OF 100 MM (4") FOR ALL BEAMS.

5.19.

NO SAWN LUMBER SHALL BE NOTCHED OR DRILLED IN THE FIELD WITHOUT THE PERMISSION OF THE CONSULTANT.

4.

ALTERATIONS AND/OR CONNECTIONS TO EXISTING STRUCTURE

4.1.

INSPECT THE EXISTING BUILDING AND BECOME THOROUGHLY FAMILIAR WITH THE EXISTING CONDITIONS.

4.2.

PRIOR TO PROCEEDING WITH THE WORK, DETERMINE THE EXACT FOUNDING ELEVATIONS OF EXISTING FOOTINGS ADJACENT TO THE NEW WORK. REPORT THESE FINDINGS TO THE CONSULTANT

4.3.

MAKE GOOD THE EXISTING WORK.

F. QUALITY CONTROL

1.

GENERAL

1.1.

IMPLEMENT A SYSTEM OF QUALITY CONTROL TO ENSURE THAT THE MINIMUM STANDARDS SPECIFIED HEREIN ARE ATTAINED.

1.2.

BRING TO THE ATTENTION OF THE CONSULTANT ANY DEFECTS IN THE WORK OR DEPARTURES FROM THE CONTRACT DOCUMENTS, WHICH MAY OCCUR DURING CONSTRUCTION. THE CONSULTANT WILL DECIDE UPON CORRECTIVE ACTION AND GIVE RECOMMENDATIONS IN WRITING.

1.3.

THE CONSULTANT'S GENERAL REVIEW DURING CONSTRUCTION AND INSPECTION AND TESTING BY INDEPENDENT INSPECTION AND TESTING AGENCIES REPORTING TO THE CONSULTANT ARE BOTH UNDERTAKEN TO INFORM THE OWNER/CLIENT OF THE CONTRACTOR'S PERFORMANCE AND SHALL IN NO WAY AUGMENT THE CONTRACTOR'S QUALITY CONTROL OR RELIEVE THE CONTRACTOR OF CONTRACTUAL RESPONSIBILITY.
2.

NOTIFICATION

2.1.

PRIOR TO COMMENCING SIGNIFICANT SEGMENTS OF THE WORK, GIVE THE CONSULTANT AND INDEPENDENT INSPECTION AND TESTING COMPANIES APPROPRIATE NOTIFICATION (MINIMUM 24 HOURS) SO AS TO AFFORD THEM REASONABLE OPPORTUNITY TO REVIEW THE WORK. FAILURE TO MEET THIS REQUIREMENT MAY BE CAUSE FOR THE CONSULTANT TO CLASSIFY THE WORK AS DEFECTIVE.
3.

INSPECTION AND TESTING

3.1.

THE CONSULTANT WILL APPOINT AN INDEPENDENT INSPECTION AND TESTING COMPANY TO MAKE INSPECTIONS OR PERFORM TESTS AS THE CONSULTANT DIRECTS. THE INDEPENDENT INSPECTION AND TESTING COMPANIES SHALL BE RESPONSIBLE ONLY TO THE CONSULTANT AND SHALL MAKE ONLY SUCH INSPECTIONS OR TESTS AS THE CONSULTANT MAY DIRECT.
4.

DEFECTIVE MATERIALS AND WORK

4.1.

WHERE EVIDENCE EXISTS THAT DEFECTIVE WORK HAS OCCURRED OR THAT WORK HAS BEEN CARRIED OUT INCORPORATING DEFECTIVE MATERIALS, THE CONSULTANT MAY HAVE TESTS, INSPECTIONS OR SURVEYS PERFORMED, ANALYTICAL CALCULATIONS OF STRUCTURAL STRENGTH MADE, AND THE LIKE, IN ORDER TO HELP DETERMINE WHETHER THE WORK MUST BE CORRECTED OR REPLACED. TESTS, INSPECTIONS OR SURVEYS OR CALCULATIONS CARRIED OUT UNDER THESE CIRCUMSTANCES WILL BE MADE AT THE CONTRACTOR'S EXPENSE, REGARDLESS OF THEIR RESULTS, WHICH MAY BE SUCH THAT, IN THE CONSULTANT'S OPINION, THE WORK MAY BE ACCEPTABLE.

4.2.

ALL TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2006 BUILDING CODE (ONTARIO REGULATION 350/06, EXCEPT WHERE THIS WOULD, IN THE CONSULTANT'S OPINION, CAUSE UNDUE DELAY OR GIVE RESULTS NOT REPRESENTATIVE OF THE REJECTED MATERIAL IN PLACE. IN THIS CASE, THE TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARDS GIVEN BY THE CONSULTANT.

4.3.

MATERIALS OR WORK, WHICH FAIL TO MEET SPECIFIED REQUIREMENTS, MAY BE REJECTED BY THE CONSULTANT WHENEVER FOUND AT ANY TIME PRIOR TO FINAL ACCEPTANCE OF THE WORK REGARDLESS OF PREVIOUS INSPECTION. IF REJECTED, DEFECTIVE MATERIALS OR WORK SHALL BE PROMPTLY REMOVED AND REPLACED OR REPAIRED TO THE SATISFACTION OF THE CONSULTANT, AT NO EXPENSE TO THE OWNER

LIST OF STRUCTURAL DRAWINGS

SHEET No.	SHEET TITLE
S1.01	GENERAL NOTES
S1.02	TYPICAL DETAILS
S2.01	REAR DECK FOUNDATION AND FRAMING PLANS
S4.01	SECTIONS AND DETAILS

KEY PLAN



LEGEND:



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Project No. 18-1011

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1	ISSUED FOR PERMIT	OCT. 2, 2017
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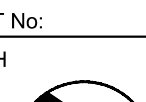
KEVIN & ANNE-MARIE TICE  
3077 LAKESHORE RD.

PROJECT TITLE:

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BURLINGTON, ON



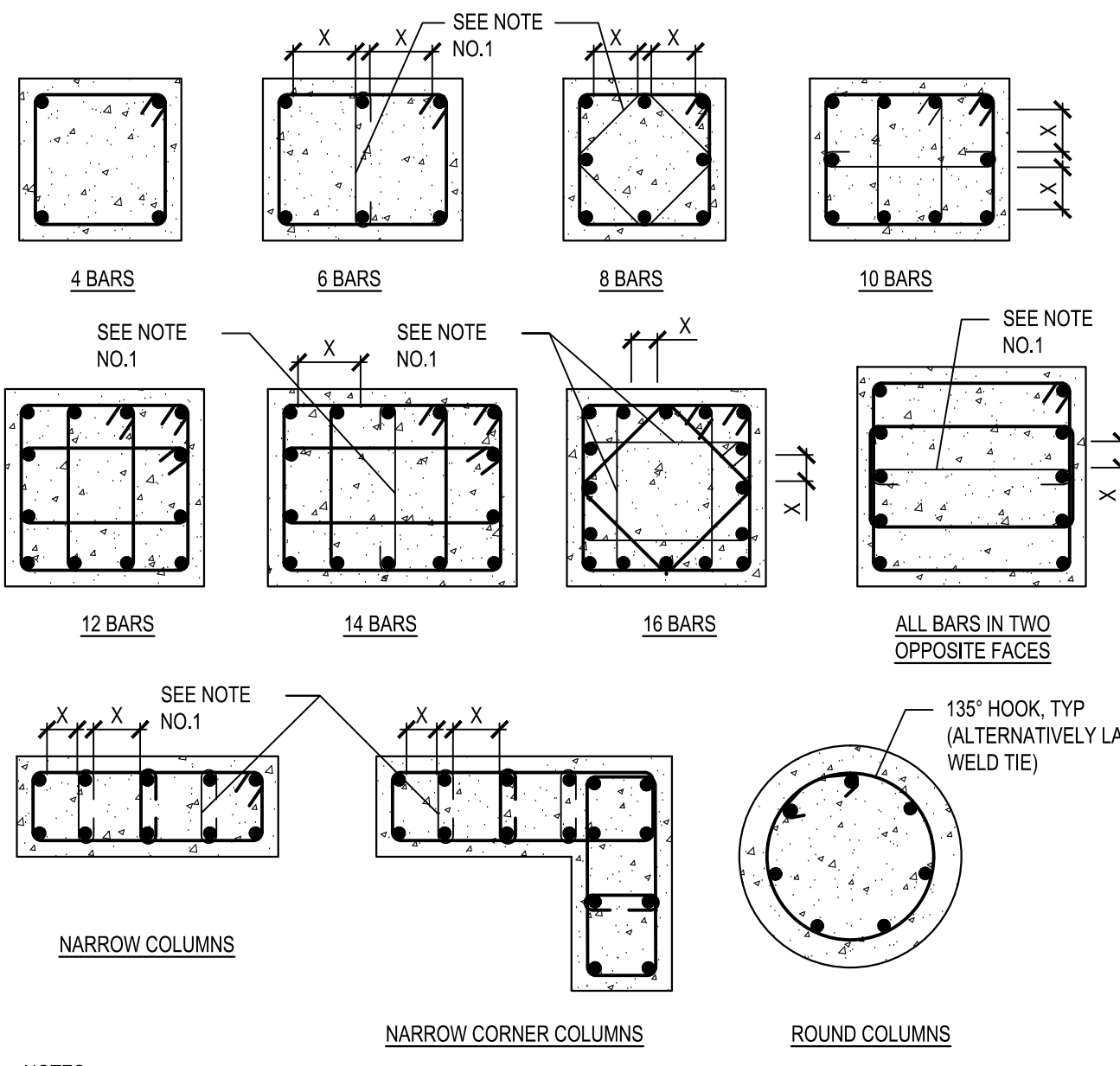
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GENERAL NOTES

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STRUCTURAL ABBREVIATIONS				TD-G01	
A BOLT	ANCHOR BOLT	E-W	EAST WEST	NTS	NOT TO SCALE
ADJ	ADJUSTABLE	EW	EACH WAY	OF	OUTSIDE FACE
AESS	ARCHITECTURAL EXPOSED	EXT	EXTERIOR	OPEN	OPENING
	STRUCTURAL STEEL	f <sub>c</sub>	28 DAYS CONCRETE	OWSJ	OPEN WEB STEEL JOIST
AFF	ABOVE FINISHED FLOOR		COMPRESSIVE STRENGTH	PI	AXIAL FORCE (FACTORED)
AIFB	ASPHALT IMPREGNATED	FDN	FOUNDATION	PC	PRECAST
	FIBREBOARD	FF	FAR FACE	PL	PLATE
ALT	ALTERNATE	FIN	FINISHED	PLF	POUNDS PER LINEAR FOOT
ARCH	ARCHITECTURAL	FL	FLOOR	PROJ	PROJECTION
ASL	ADDITIONAL ACCUMULATED SNOW LOAD	ft	FOOT, FEET	PSF	POUNDS PER SQUARE FOOT
@	AT	FTG	FOOTING	PT	PRESSURE TREATED
B, BOTT	BOTTOM	Fy	YIELD STRENGTH	RD	ROOF DRAIN
B/B	BACK TO BACK	GA	GAUGE	RI	REACTION (FACTORED)
BEW	BOTTOM EACH WAY	GALV	GALVANIZED	RAD	RADIUS
BH	BOREHOLE	GEN	GENERAL	REINF	REINFORCED, REINFORCEMENT
BLL	BOTTOM LOWER LAYER	HEF	HORIZONTAL EACH FACE	REF	REFERENCE
BLDG	BUILDING	HI	HORIZONTAL FORCE (FACTORED)	RE	RIGHT END
BM	BEAM	HH	HOOK EACH END	REQD	REQUIRED
BPL	BEARING/BASE PLATE	HIF	HORIZONTAL INSIDE FACE	REV	REVISION, REVISED
BRDG	BRIDGING	HOF	HORIZONTAL OUTSIDE FACE	R/W	REINFORCED WITH
BUL	BOTTOM UPPER LAYER	H, HORZ	HORIZONTAL	SDF	STEP DOWN FOOTING
c	CAMBER	HSC	HORIZONTALLY SLOTTED CONNECTION	SECT	SECTION
C	EPOXY COATED	HSS	HOLLOW STEEL SECTION	SIM	SIMILAR
c/c, o/c	CENTRE TO CENTRE	IF	INSIDE FACE	SL	SLAB
CA, CB	COLUMN ABOVE,	IN	INCHES)	SOG	SLAB ON GRADE
CANT	COLUMN BELOW	INT	INTERIOR	SPDD	STANDARD PROCTOR DRY DENSITY
CF	CANTILEVER	JT	JOINT	STD	STRAIGHT
CJ	COMPRESSIVE FORCE (FACTORED)	K	KIP, 1000 LBS	STIFF	STIFFENER
CL	CONTROL JOINT	K-ft	KIP FEET	STIRRUP	STIRRUP
CL @	CLEAR	kg	KILOGRAM(S)	STRUCT	STRUCTURAL
COL	CENTRELINE	KLF	KIPS PER LINEAR FOOT	STD	STANDARD
COL	COLUMN	KN	KILONEWTON	SQ	SQUARE
COMP	COMPOSITE	KN-m	KILONEWTON METRE	T	TOP
CONC	CONCRETE	KN/m	KILONEWTON PER METRE	TT	TENSILE FORCE (FACTORED)
CONT	CONTINUOUS	kPa	KILOPASCAL	TEMP	TEMPORARY, TEMPERATURE
CW	COMPLETE WITH	KSF	KIPS PER SQUARED FOOT	TEW	TOP EACH WAY
DEMO	DEMOLITION	KSI	KIPS PER SQUARED INCH	TJ	TIE JOIST
DET	DETAIL	L	SINGLE ANGLE	TLL	TOP LOWER LAYER
DIA, Ø	DIAMETER	LE	LEFT END	TMF	TORSIONAL MOMENT (FACTORED)
DIAG	DIAGONAL	LG	LONG	TOD	TOP OF DECK
DIM	DIMENSION	LL	LIVE LOAD, LOWER LAYER	TOS	TOP OF STEEL/SLAB
DL	DEAD LOAD	LLH	LONG LEG HORIZONTAL	TRANS	TRANSVERSE
DP	DEEP	LLV	LONG LEG VERTICAL	TUL	TOP UPPER LAYER
DWG(S)	DRAWING(S)	m	METRE	TYP	TYPICAL
DWL(S)	DOWEL(S)	MC	MOMENT CONNECTION	UL	UPPER LAYER
DN	DOWN	MECH	MECHANICAL	U/S	UNDERSIDE
EA	EACH	Mf	MOMENT (FACTORED)	V, VERT	VERTICAL
EE	EACH END	ML	MIDDLE LAYER	VI	VERTICAL SHEAR FORCE (FACTORED)
EF	EACH FACE	mm	MILLIMETRE	VPF	VERTICAL BRACED FRAME
ELEC	ELECTRICAL	MPa	MEGAPASCAL	VEF	VERTICAL EACH FACE
EL	ELEVATION	Mxf	BENDING MOMENT	VIF	VERTICAL INSIDE FACE
ELEV	ELEVATOR		ABOUT x-x AXIS (FACTORED)	VOF	VERTICAL OUTSIDE FACE
EMBED	EMBEDMENT	Myf	BENDING MOMENT	VSC	VERTICALLY SLOTTED CONNECTION
EQ	EQUAL		ABOUT y-y AXIS (FACTORED)	W	WIDE FLANGE BEAM
ES	EACH SIDE	NF	NEAR FACE	WT	WEIGHT, STRUCTURAL TEE
EX, EXIST	EXISTING	NIC	NOT IN CONTRACT	WWF	WELDED WIRE FABRIC OR WELDED WIDE FLANGE
EJ, EXP JT	EXPANSION JOINT	N-S	NORTH-SOUTH	W.P.	WORKING POINT

ARRANGEMENT OF REINFORCING IN COLUMNS		TD-C31
		
<p>NOTES</p> <ol style="list-style-type: none"> <li>1. TIE VERTICALS WITH TIES SHOWN DASHED WHEN "X" IS OVER 150 mm.</li> <li>2. MINIMUM CLEAR DISTANCE BETWEEN VERTICAL BARS:             <ol style="list-style-type: none"> <li>a. 1.4 TIMES VERTICAL BAR DIAMETER.</li> <li>b. 1.4 TIMES THE MAX SIZE OF COURSE AGGREGATE.</li> <li>c. 30 mm</li> </ol> </li> <li>3. SEE DRAWINGS FOR OTHER TIE ARRANGEMENTS.</li> <li>4. WHEN SEISMIC HAZARD INDEX, <math>I_e F_a S_a(0.2)</math>, IS LESS THAN 0.35 SINGLE TIES WITH ONE 90° AND ONE 135° HOOK MAY BE USED PROVIDED THE 90° HOOK IS MIN 100 mm LONG AND SUCCESSIVE TIES ENGAGING THE SAME BARS ARE ALTERNATED END FOR END.</li> </ol>		

REINFORCEMENT DEVELOPMENT LENGTHS

TD-C11

TABLE 1 TENSION DEVELOPMENT LENGTH (mm)

BAR SIZE	f <sub>c</sub>				
	20 MPa	25 MPa	30 MPa	35 MPa	40 MPa
10	320	300	300	300	300
15	480	430	390	370	340
20	640	580	530	490	460
25	1010	900	820	760	710
30	1210	1080	990	910	850
35	1410	1260	1150	1060	1000
45	1820	1620	1480	1370	1290
55	2220	1980	1810	1680	1570

TABLE 2 TENSION LAP SPLICE (CLASS B) LENGTH (mm)

BAR SIZE	f <sub>c</sub>				
	20 MPa	25 MPa	30 MPa	35 MPa	40 MPa
10	420	380	340	315	300
15	630	560	510	485	445
20	840	755	690	640	600
25	1315	1155	1035	955	885
30	1575	1385	1245	1135	1055
35	1840	1640	1480	1360	1240
45					
55	LAP SPLICES NOT PERMITTED				

TABLE 3 DEVELOPMENT LENGTH (mm) FOR STANDARD HOOKS

BAR SIZE	f <sub>c</sub>				
	20 MPa	25 MPa	30 MPa	35 MPa	40 MPa
10	155	150	150	150	150
15	240	210	190	175	170
20	315	280	260	240	225
25	390	350	320	295	280
30	470	420	385	360	330
35	550	430	450	415	385
45	977	874	798	739	691
55	1261	1128	1030	953	892

TABLE 4 COMPRESSION DEVELOPMENT LENGTH (mm)

BAR SIZE	f <sub>c</sub> = 20 MPa		f <sub>c</sub> = 25 MPa		f <sub>c</sub> > 30 MPa	
	20 MPa	25 MPa	25 MPa	30 MPa	30 MPa	40 MPa
10	210		200		200	
15	320		290		260	
20	430		380		350	
25	540		480		440	
30	640		580		530	
35	750		670		620	
45	970		860		790	
55	1180		1060		970	

TABLE 5 COMPRESSION LAP SPLICE LENGTH (mm)

BAR SIZE	USUAL CONFINEMENT	
	90° HOOK	180° HOOK
10	300	
15	440	
20	580	
25	730	
30	880	
35	1020	

TABLE 6 STANDARD HOOK DIMENSION FOR BLACK REINFORCING

BAR SIZE	400R OR 500R		400W OR 500W	
	90° HOOK (mm)	180° HOOK (mm)	90° HOOK (mm)	180° HOOK (mm)
10	180	140	180	130
15	260	180	250	170
20	310	220	300	200
25	400	280	400	280
30	510	400	490	350
35	610	480	590	430
45	790	680	770	620
55	1030	900	1010	830

NOTE: 45M AND 55M BARS SHALL BE SPLICED WITH MECHANICAL CONNECTORS.

REFER TO REINFORCING STEEL MANUAL OF STANDARD PRACTICE FOR MORE INFORMATION.

KEY PLAN



LEGEND:



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CLIENT:

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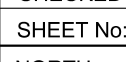
PROJECT TITLE:

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BURLINGTON, ON



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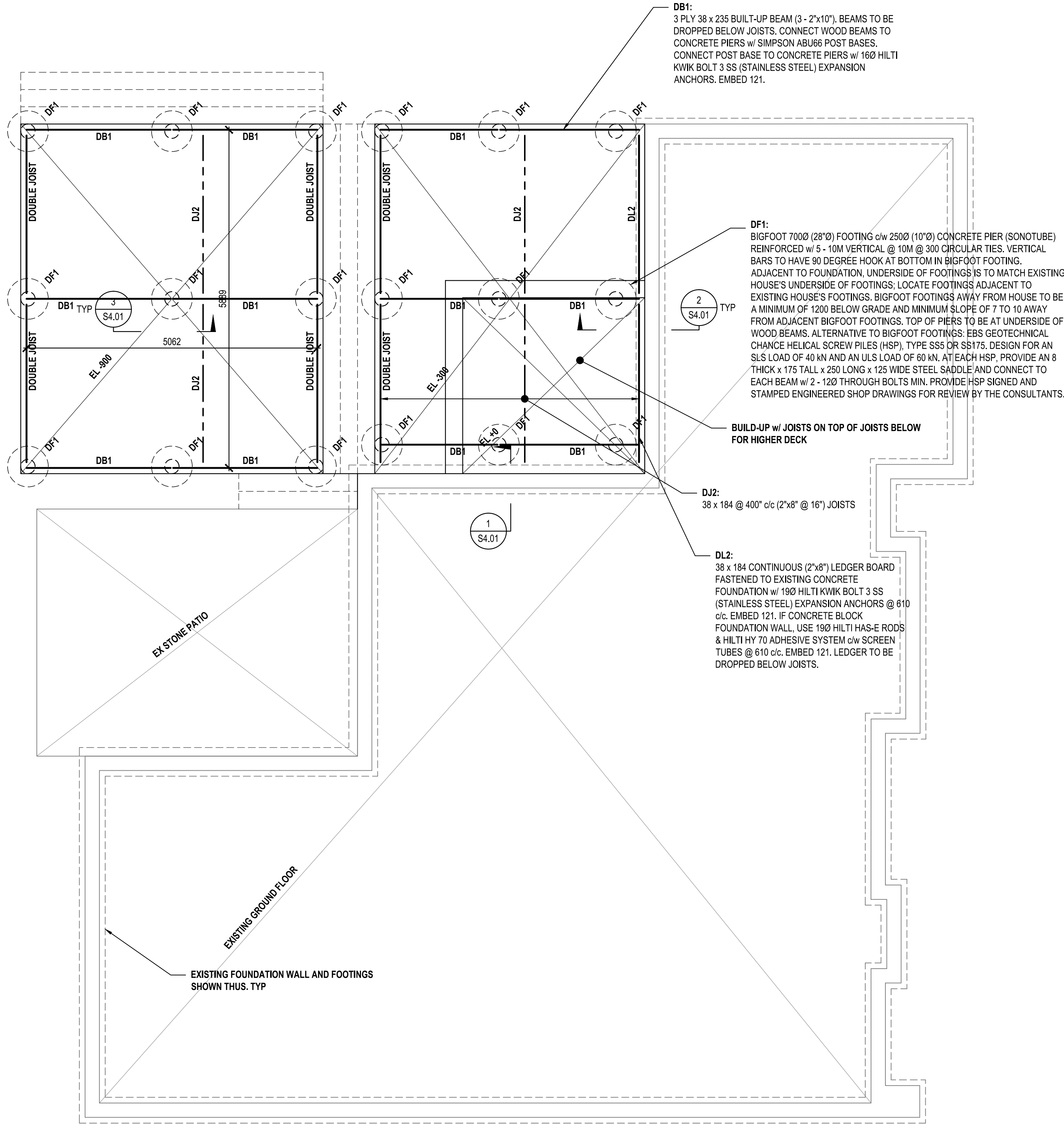
TYPICAL DETAILS

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	DESIGN BY:	CN	
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REAR DECK FOUNDATION AND FRAMING PLAN

NOTES:

- TOP OF EXISTING INTERIOR GROUND FLOOR IS TO BE +0 BELOW EXISTING GROUND FLOOR ELEVATION +0. AREAS CROSS AND NOTED ARE TO BE READ FROM THE GROUND FLOOR ELEVATION +0. REFER TO LANDSCAPING DRAWINGS FOR FINAL TOP OF DECK ELEVATIONS.
- LIVE LOADS ARE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN:

DECK	1.90 kPa
------	----------
- DEAD LOADS ARE:

DECKING	0.20 kPa
STRUCTURE	0.30 kPa
MISC.	0.30 kPa
- REFER TO GENERAL NOTES FOR SOIL BEARING CAPACITY.
- ALL WOOD FRAMING TO BE SPF (SPRUCE-PINE-FIR) No.1/No.2 AND PRESSURE TREATED.
- OWNER TO CHOOSE DECK BOARDS (38 (1 1/2") PLASTIC OR WOOD BOARDS).
- REFER TO ONTARIO BUILDING CODE SUPPLEMENTARY STANDARD SB-7 FOR GUARDS FOR HOUSING AND SMALL BUILDINGS. CONTRACTOR TO CHOOSE AND INSTALL GUARDS AS PER GUIDELINES.

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SHEET TITLE:

REAR DECK FOUNDATION AND  
FRAMING PLAN

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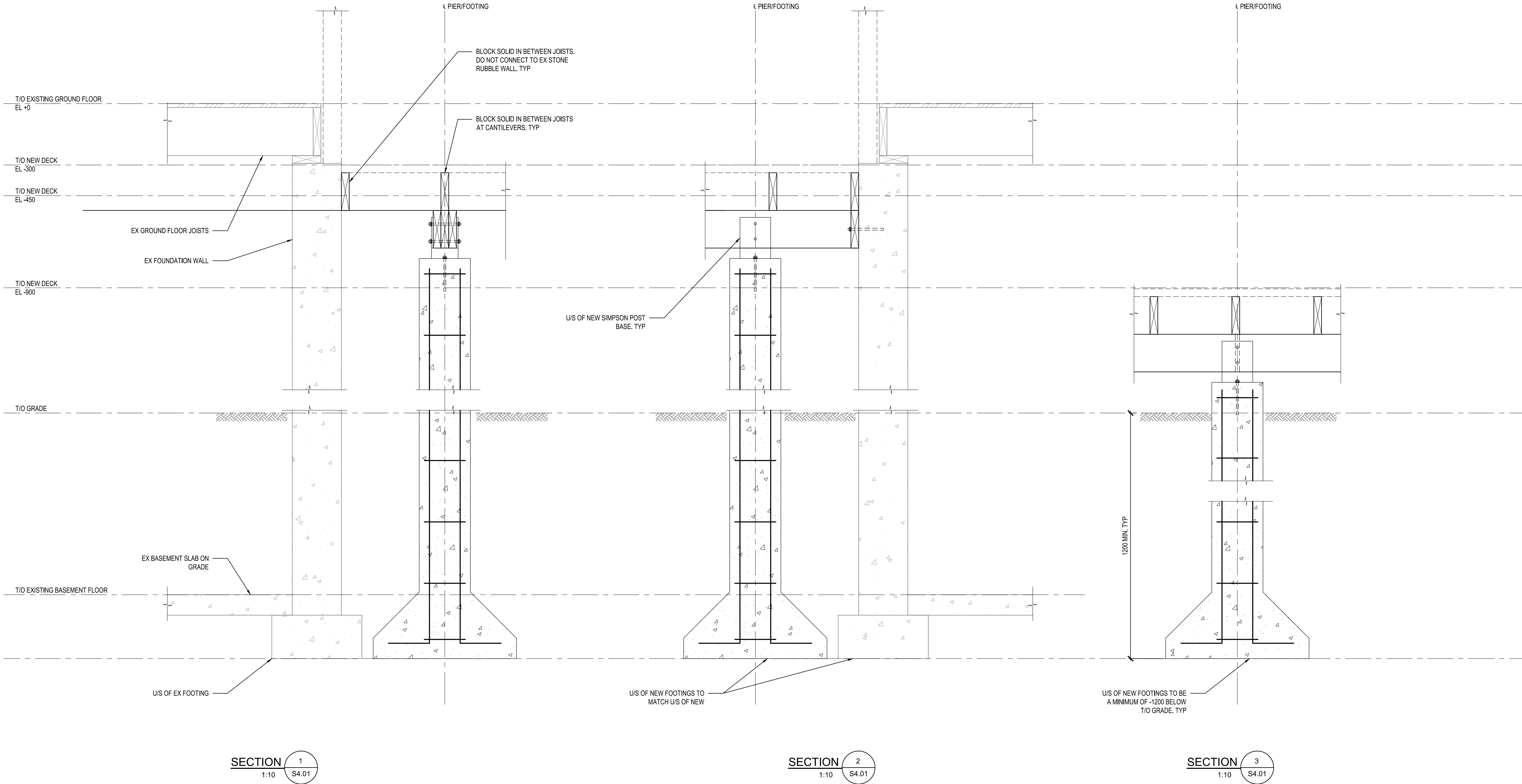
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SECTION 1  
1:10 S4.01

SECTION 2  
1:10 S4.01

SECTION 3  
1:10 S4.01

KEY PLAN



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CLIENT:

KEVIN & ANNE-MARIE TICE  
3077 LAKESHORE RD.

PROJECT TITLE:

3077 LAKESHORE RD.  
BURLINGTON, ON



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SHEET TITLE:

SECTIONS AND DETAILS

DWG No: S4.01

SCALE: AS SHOWN

PROJECT NO: 16-034

DRAWN BY: CN

DESIGN BY: CN

CHECKED BY: CN

SHEET No:

NORTH

